

# Basic Concepts of Groundwater Hydrology

## Objectives

1. Review important terminology and concepts, so everyone has sufficient background to absorb course materials.
2. Highlight the topics that are relevant to SW-GW interaction.

## Darcy's Law

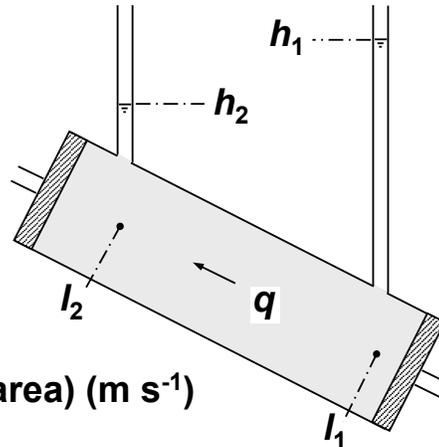
$$dh = h_2 - h_1 \quad \text{hydraulic head}$$

$$dl = l_2 - l_1$$

$$q = -K \frac{dh}{dl}$$

$q$ : specific discharge (= flow rate / area) ( $\text{m s}^{-1}$ )

$K$ : hydraulic conductivity ( $\text{m s}^{-1}$ )



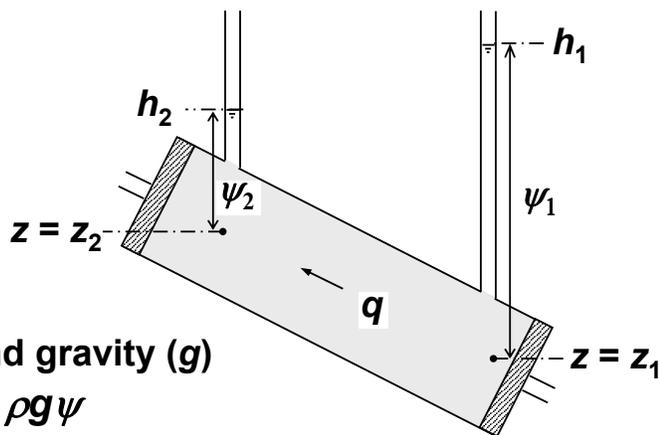
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Hydraulic head has two parts.

$$h_1 = z_1 + \psi_1$$

$$h_2 = z_2 + \psi_2$$

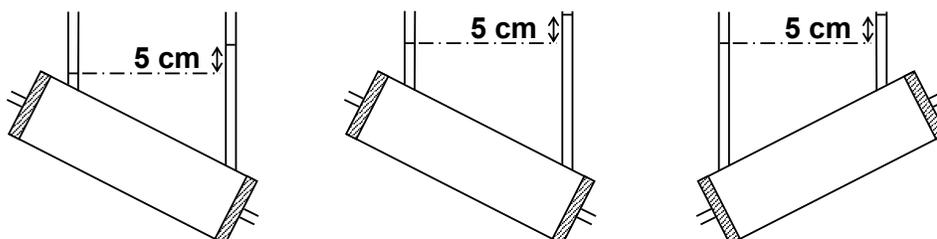
elevation      pressure head



Multiplying  $h$  by density ( $\rho$ ) and gravity ( $g$ )

$$\rho gh = \rho gz + \rho g\psi$$

$$\text{hydraulic potential} = \rho gz + P \quad \text{gravitational potential} + \text{pressure (J m}^{-3}\text{)}$$

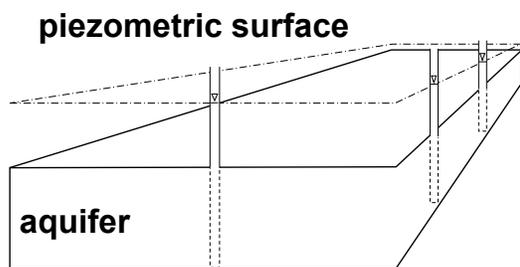


All tubes have the same flow rate – flow is driven by the hydraulic gradient, not by the absolute values of the head.

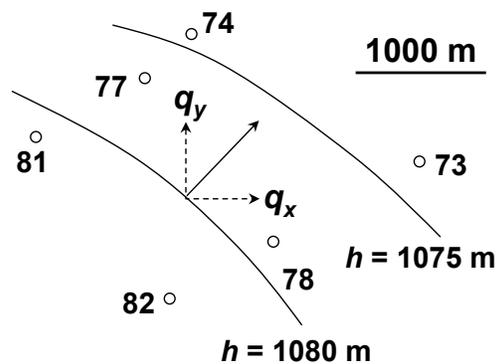
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## Darcy's Law in a confined aquifer



$$q_x = -K \frac{\partial h}{\partial x} \quad q_y = -K \frac{\partial h}{\partial y}$$



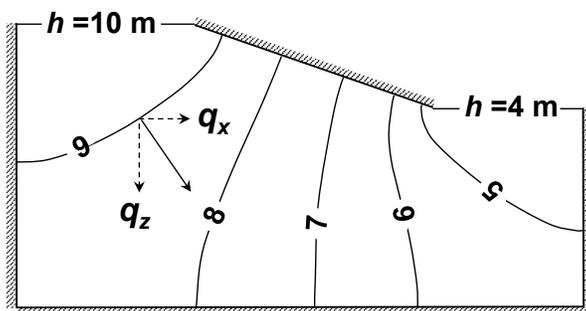
Contour lines of  $h$  are called 'equipotentials'. Flow vectors are normal to equipotentials in homogeneous, isotropic aquifers.

Suppose that the aquifer consists of the medium sand and temperature is  $5^\circ\text{C}$ .

Magnitude of the flow vector  $\vec{q} = (q_x, q_y)$ ?

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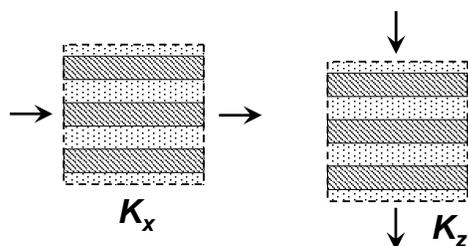
## Darcy's Law in a vertical cross section



$$q_x = -K \frac{\partial h}{\partial x} \quad q_z = -K \frac{\partial h}{\partial z}$$

## Layering and anisotropy of $K$

Suppose a layered sediment: sand and clay

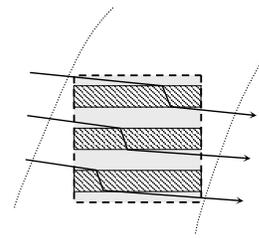


$$q_x = -K_x \frac{\partial h}{\partial x} \quad q_z = -K_z \frac{\partial h}{\partial z}$$

Hydraulic conductivity is dependent on direction: anisotropy.

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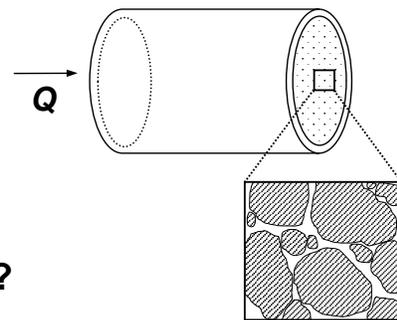
Flow lines may not be normal to hydraulic head contour lines in anisotropic materials.



Are layered sediments common near lakes and rivers?

**Specific discharge and average linear velocity**

Suppose  $Q$  ( $\text{m}^3 \text{s}^{-1}$ ) of water is flowing through a sand column with an area  $A$  ( $\text{m}^2$ ).



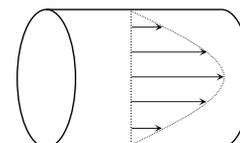
Specific discharge  $q$  ( $\text{m s}^{-1}$ ) = ?

Is this equal to the average flow velocity ( $v$ )?

porosity =  $n_p$

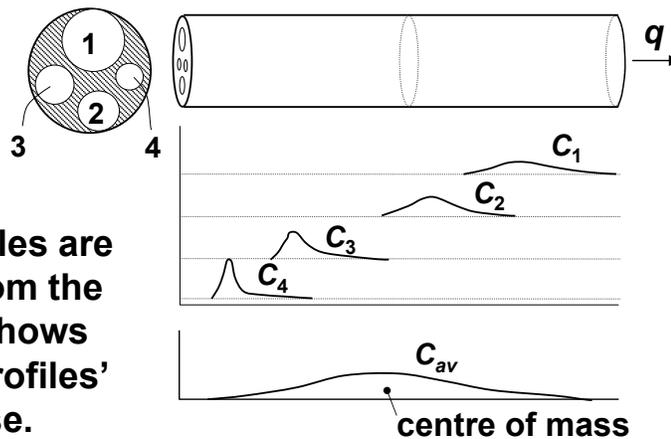
Actual flow velocity varies within a pore 'tube'.

Why?



Flow velocity also varies among different-size pores.

Why?

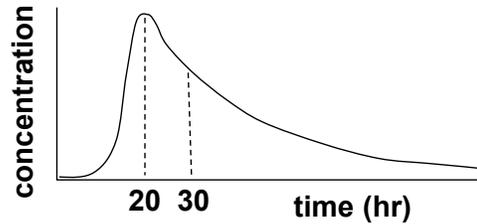
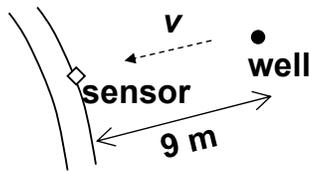


Suppose that solute molecules are instantaneously released from the left end. The middle graph shows theoretical concentration 'profiles' at some time after the release.

What we can 'measure' by sampling the column is the bottom profile, showing the average concentration of all pores. The centre of solute mass travel at the linear average velocity ( $v$ ).

The spreading is called 'dispersion'.

Understanding of dispersion is important for the interpretation of tracer test data used in SW-GW interaction studies.

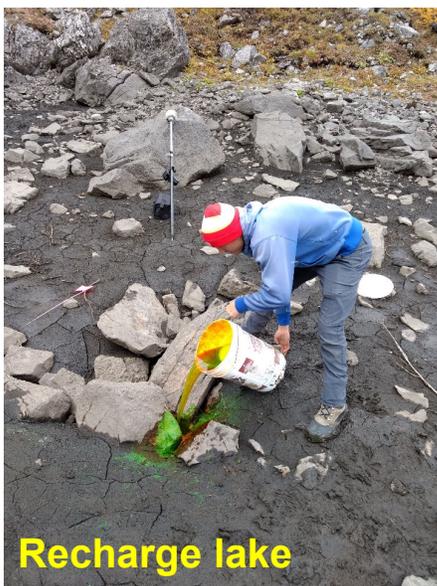


Suppose a 'conservative' tracer was released from a well located 9-m upslope from a stream. Average linear velocity ( $v$ ) ?

Hydraulic gradient between the well and the stream is 0.02. Horizontal hydraulic conductivity ( $K$ )?

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## Fluorescein (a.k.a. Uranine) Tracer Test Fortress Ski Area, Kananaskis, AB Groundwater flow through coarse moraine



~150m  
→



Travel time = 230 min,  $v = 0.011 \text{ m s}^{-1}$ .  
→  $K$  of moraine  $\sim 0.1 \text{ m s}^{-1}$

He and Hayashi (2023, *Hydro. Process.*, in press)

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